

## 3D FE and 2DOF simulations of ground shock experiments – Reflection pressure time history dependency due to the structure’s stiffness and mass

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This paper simulates, by using 3D Finite Element (FE) Autodyn and simple two degree of freedom (2DOF) model, experiments from 1980s conducted by S. Hultgren, FORTF, where Hultgren studied the structural response of a well-defined structure; a suspended piston-spring system buried in sand subjected for ground shock from an explosive charge. The experiments showed that if the suspended mass of the piston is increased the initial reflected pressure also increases. Similarly, the experiments showed that if the stiffness of the suspended piston is increased the reflected pressure time history increases for the latter part of the reflected pressure curve. The first aim of the FE simulations was to better understand the physics of the observed experimental results. Based on this, the second aim was to find a methodology that can use simplified relationships for ground shock prediction, from e.g. ConWep, in combination with simplified models such as 2DOF, to predict the structural response of e.g. a buried concrete wall.

The FE simulation models were generated in Autodyn-3D, where the sand was modelled with Euler cells and the piston, spring, and cylinder were modelled with Lagrange element. The sand was modelled with an Equation of State (EOS) designed for porous soils. The FE simulation results confirm the trend found in the experimental results. They also imply that the mass from the interacting ground material is greatly affecting the response of an underground structure subjected to impulse loading. The simple 2DOF model confirms some of the main behaviour found in the FE results and experiments, such as the initial collision effect only depends on mass and that total spring deflection gave fairly well agreement with simulations.

### INTRODUCTION

The Swedish Civil Contingencies Agency (MSB) is responsible for the building regulations of the Swedish civil defence shelters. There are specific regulations for how the defence shelters are planned, built, equipped and maintained [1]. One of many regulations state what load level the shelters should be able to withstand: “The effect of a pressure wave corresponding to that produced by a 250 kg GP-bomb with 50 weight per cent TNT which burst freely outside at a distance of 5.0 meters from the outside of the shelter during free pressure release”. However, many of the shelters are designed as basements below ground surface. Therefore, more knowledge on how the ground shock propagates and attenuates during the scaled distances of 0.1 to 10 kg/m<sup>1/3</sup> and effects on buried shelters, is needed.

During the Second World War, extensive experiment series and research were conducted on ground shock generated by high explosives [2]. This early work functions as a foundation for understanding the behaviour of how the shock waves propagates and attenuates in earth media. In [2], it is stated that the pressure in the soil from the detonation of an explosive charge is propagated by a plastic wave which is characterized by a continuous change of shape and of duration with distance from the charge, see Fig. 1 for schematic illustration. Close to the source the shape will be sharp and after a distance it will start to attenuate and the wave will change shape.

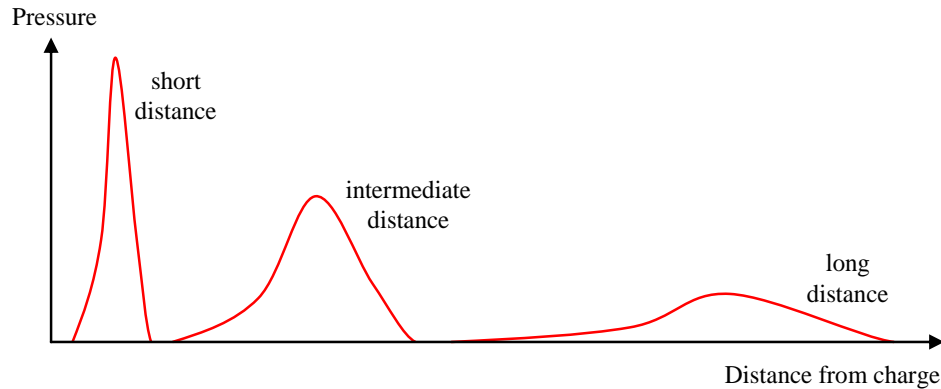


Fig. 1 Schematic illustration of how the free field ground shock wave change shape with distance during propagation in a compactable soil.

When it comes to the structural response from the propagated pressure wave from ground shock it is not as obvious as when it comes to the airblast case. When predicting the structural response from airblast it is usually sufficient to separate the simulation in two parts:

- 1) Calculation of the pressures acting on rigid body, shaped as the studied structure, from e.g. an airblast simulation including the explosive and air formulation in a multi material Euler grid.
- 2) Determine the structural response by applying the calculated pressure time histories from 1) on the deformable structure modelled by e.g. shell elements.

This procedure is suitable for stiff structures, such as reinforced concrete structures, when subjected to air blast loading. For simple cases where the threat is directly in front of the structure step 1 above can be simplified to use empirical equations, e.g. for the incident airblast and adjust what the reflected pressure should be on the structure. However, this procedure is not applicable when it comes to the structural response due to ground shock simulations since, in ground shock, the reflected pressure actually depends on the movable mass and stiffness of the structure [3].

Hence, to determine the pressure acting on a structure, caused by ground shock, one must also know the properties of the structure. In 1979-1980 S. Hultgren at the National Fortifications Administration in Sweden, FortF, conducted experiments with a simplified structure, a buried suspended piston with mass  $m$  and stiffness  $k$ . Neglecting the influence of damping, the response of this structure can be described using a single degree of freedom system by using equation (1)

$$P_r(t) \cdot A = m \cdot a_x(t) + k \cdot d_x(t) \quad (1)$$

where  $P_r(t)$  is the reflected pressure acting on the piston surface area  $A$ ,  $a_x$  is the acceleration of the piston mass and  $d_x$  is the displacement of the piston.

These results show the same trend on how the reflecting pressure builds up by mass inertia and spring stiffness, see Fig. 2. Experimental results from reports [3]-[4] can be summed up in two points:

- If the structure has a high mass but low stiffness, then the reflected pressure time history results in a large first peak and a low second peak.
- If the structure has a low mass but high stiffness, the reflected pressure time history gets a low initial peak and a high second peak.

In 1985, experiments were conducted for buried deformable reinforced concrete walls with similar results [5]. This paper compare the experimental results from [3]-[4] with simulations carried out in AUTODYN-3D [6] and a simplified 2DOF model. Earlier comparisons have also been carried out in AUTODYN-2D [20]. These early simulations confirmed the main trends from the experiments. However, the symmetric 2D simulations in [20] included uncertainties such as improper air blast release due to buried explosive and uncertainty in actual reflected pressure measurements on the 2D piston. These uncertainties have been removed in the 3D simulation model presented in this paper.

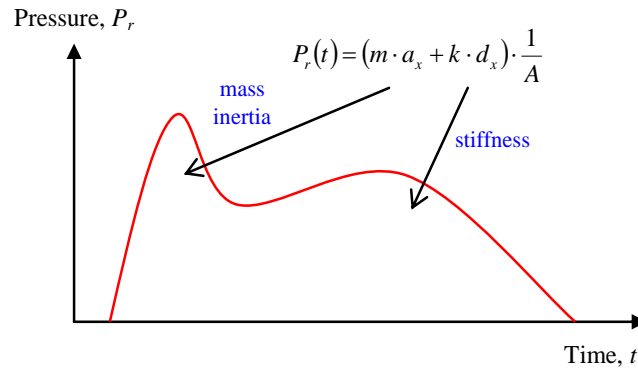


Fig. 2 Schematic illustration of how the reflection pressure  $P_r(t)$  is made up of mass inertia  $m \cdot a_x$  and stiffness  $k \cdot d_x$  of a buried structure.

The paper is organized as follows: The section EXPERIMENTAL SETUP, describes how the experiments were conducted and what parameters were varied. In section FE SIMULATION MODEL it is shown how the Euler and Lagrange elements were designed in AUTODYN-3D and what material models were used. In section 2DOF MODEL it is shown what parameters and initial conditions were used in the 2DOF model. In the section EXPERIMENTAL RESULTS the experimental results from [3]-[4] is presented and in SIMULATION RESULTS a comparison is made with the results obtained from simulations in AUTODYN. Finally, the section CONCLUSIONS AND FUTURE WORK concludes the findings from simulations and proposes suggestions for future work.

### EXPERIMENTAL SETUP

The experiments about the reflected pressure on a buried single degree of freedom system were conducted in sand. Both the charge of TNT with weight 0.5 kg and the suspended piston was buried 1 m, and the distance between the charge and the piston was set to 1 m. Unfortunately, the in-situ density of the sand was not measured nor the actual water content. However, in [3] it is mentioned that the sand was well compacted and not saturated. The experimental set up is shown in Fig. 3.

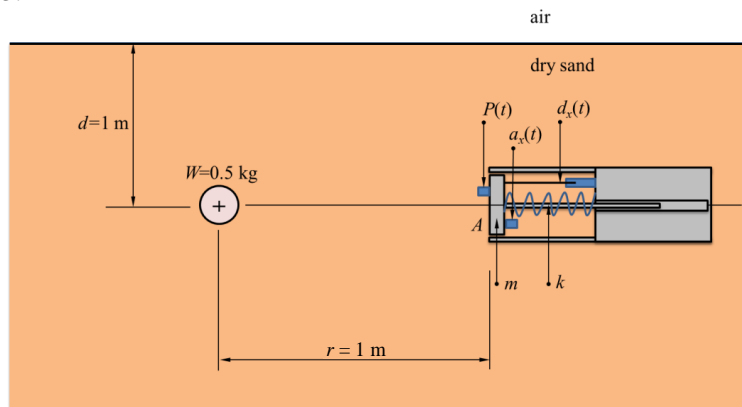


Fig. 3 Principal sketch of the experimental setup of [3]: buried piston with suspended mass  $m$  and stiffness  $k$ .

The main cylinder body consisted of a circular steel tube of 1.18 m in length and 0.36 m in outer diameter. The cylinder wall thickness was 8 mm and the diameter of the piston surface was 0.34 m. In order to increase the weight of the cylinder tube lead pieces were bolted to its inside; thus, increasing the total weight of the cylinder body to a total of 295 kg. The piston was movable on ball bearings through an axis and the stiffness was obtained with a helical spring.

The piston's movable mass was made of a removable plate and by changing plates with different thickness the suspended mass  $m$  of the piston was varied [5.2, 10.6, 24.7, and 58.8. Further, by changing the helical spring the stiffness  $k$  was also varied [0.1, 0.5, and 1.2 MN/m].

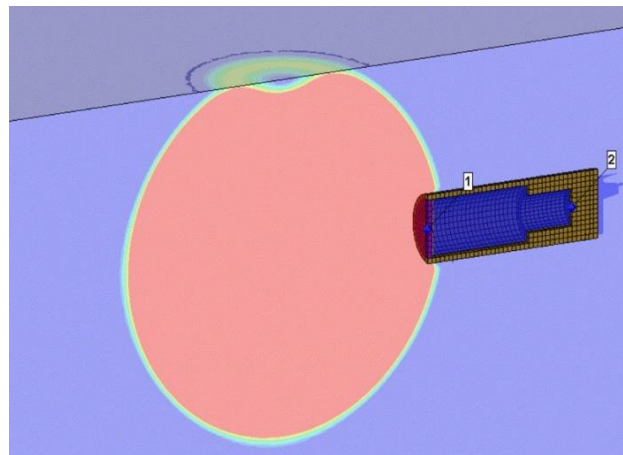
The following measurements were installed: piston accelerometer ( $a_x$ ), piston reflected pressure gauge ( $P_r$ ), and relative piston displacement ( $d_x$ ), see Fig. 3. Further details about the experimental setup, e.g. what model type of sensors was used, can be found in [3].

### FE SIMULATION MODEL

The FE model was built in AUTODYN [6] using halved 3D geometry. The 3D symmetry plane was oriented perpendicular to the ground surface cutting the cylinder in two with the piston axis lying in the symmetry plane, see Fig. 4. The size of the model was 100 m x 100 m x 50 m, which meant that the effects of reflections from the boundary could be entirely avoided for the piston response of at least 50 ms. There is a fine mesh volume 2.4 x 2.4 x 3.6 m near the cylinder with cubic cell size of 20 mm; where the longer direction is to include the whole Lagrange cylinder within the fine mesh region. The sand, TNT, air, and VOID material inside the cylinder were modelled with multi-material Euler elements. The piston plate (1) and cylinder body were modelled using rigid Lagrange elements constrained to move only in the piston direction. The spring was modelled with an ideal Hooke's law formulation where the spring force acting on the internal plate surface and the cylinder housing was proportional to the compression distance

$$F_s(t) = k \cdot (d_1(t) - d_2(t)) \quad (2)$$

where  $d_1$  and  $d_2$  are the displacement of the end positions of the spring, see markings 1 and 2, respectively, in Fig 4. The simulations were optimized for computational time by grid refinement and using re-mapping technique as more detailed described in e.g. [8]. The model was also parameterized so that charge size, distance to piston plate, piston plate mass, and spring stiffness could automatically be varied. Gravity was considered to have minimal impact on the simulation results and was hence not included in the model.



*Fig. 4 Halved 3D model, the shock wave from the 0.5kg TNT charge just reaching the piston and ground surface. The numbers 1 and 2 mark the spring start and end points.*

### Modelling of the sand

A simple way of modelling compaction materials is to define the plastic compaction curve as a pressure function of density  $P(\rho)$  and the unloading wave speed as a function of density  $c(\rho)$ , see Fig. 5 for a schematic illustration. The unloading is then approximately represented by the straight blue lines on top of the dashed unloading curves. The solid straight line marked with TMD means Theoretical Maximum Density.

Earlier work on deriving mechanical properties for dry sand from tri-axial experiments, [9] and [10] does not include an easy change on how moisture affects the input parameters of the compaction EOS. When the soil type changes in water contents, porosity, and soil skeleton, the original compaction model [9] would need a new set of input. This has been studied in [15] where a total of 16 generic soil types was generated from dry sand to fully saturated clay. However, the main deficiency of the original model [16] for the compaction EOS is that the unloading phase is too simplified to properly model the shock wave propagation and the change in shape of the pressure wave as stated in [2]. In [17] an initial study was made of what the solution of unloading wave  $c(\rho, P)$  for the dry sand would look like by using Non-Linear Programming (NLP) for numerically approximating each unloading curve. This allowed the experimental data from [9] and [10] to be extrapolated for the whole density pressure domain of interest [18].

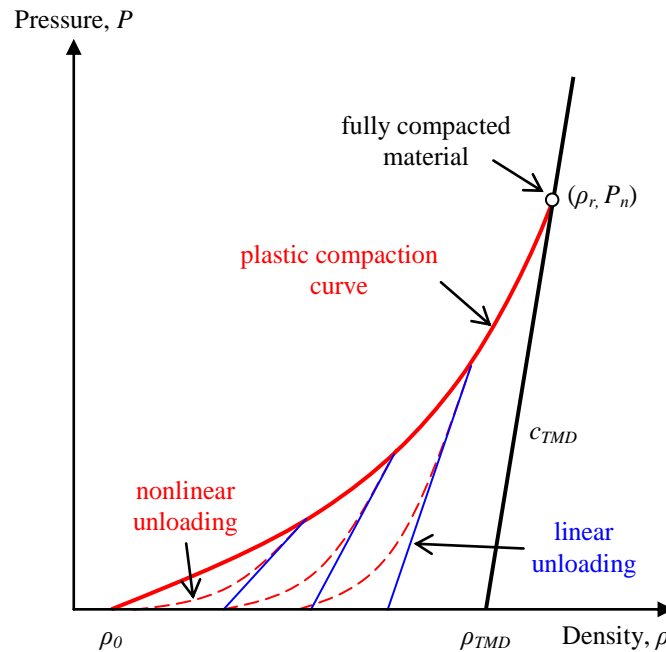


Fig. 5 Schematic illustration of an EOS compaction, where the solid line between  $(\rho_0, P_0 = 0)$  and  $(\rho_r, P_r)$  shows a plastic compaction curve, dashed red lines illustrate nonlinear unloading and straight blue lines show linear approximation with elastic unloading wave  $c(\rho)$ .

However, in this paper only the original (denoted Sjöbo) material model was used in the 3D simulations, comparison of the modified material model (ModEOS) and Sjöbo was done in earlier 2D simulations [20]. In Sjöbo material model, the unloading speed only depends on density  $c(\rho)$ . The choice of not including both material models in the study was to focus on the main dynamic behaviour related to the interaction of the sand and the buried piston. Another benefit with Sjöbo sand is that original data and EOS and strength formulation is available for all Autodyn users in the standard library. The material properties for both volume and shear behaviour, and the user subroutines for the modified EOS and modified shear strength model for AUTODYN can be found in [19].

### Modelling of the piston

The piston was made of rigid material with a constant volume, and hence its density was adjusted so that it matched the desired piston mass for each specific case. The piston was also constrained to only move in the axial direction.

### Modelling of the spring

The spring was modelled by employing a 1-element beam structure with an ideal Hooke's law spring formulation.

### Modelling of the cylinder

The cylinder was made of rigid material the total weight of the cylinder was 295 kg in the simulation.

## 2DOF MODEL

The 2DOF model simplifies the experimental setup by the following assumptions, the sand in front of the structure is assumed to be modelled as an added mass, see Fig. 6 The explosive load is converted to either a force as function of time acting on the added mass or as a mass with an initial particle velocity on the added mass. In Fig. 7., the principal system of 2DOF is shown. The spring  $k_1$  can only transfer compressive forces while spring  $k_2$  can transfer forces in both directions. The initial velocity  $v_0$  (i.e. the sand particle velocity  $U_{p0}$ ) is used for describing the movement of the sand generated by the explosion.

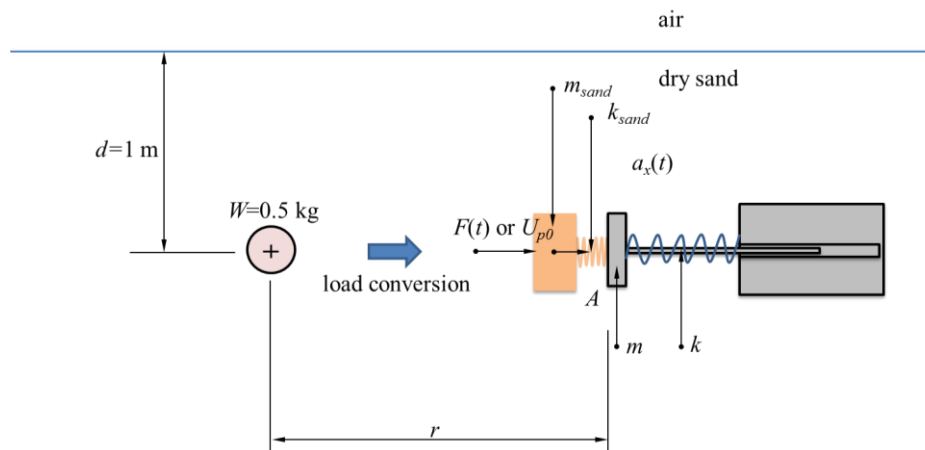


Fig. 6 Illustration of how the 2-DOF model is set up. The piston mass is connected to a second mass which is assumed to be an added mass from the sand and during compression the masses have a spring stiffness based on the bulk modulus of the sand.

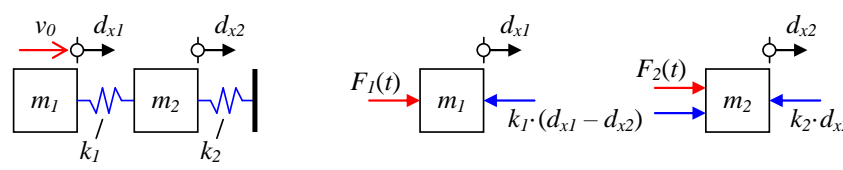


Fig. 7 Illustration of 2DOF system used. Spring  $k_1$  can only transfer compressive forces while spring  $k_2$  can transfer forces in both directions.

The equations used to solve the 2DOF system can be written as

$$\begin{bmatrix} m_1 & 0 \\ 0 & m_2 \end{bmatrix} \begin{bmatrix} a_{x1} \\ a_{x2} \end{bmatrix} + \begin{bmatrix} k_1 & -k_1 \\ -k_1 & k_1 + k_2 \end{bmatrix} \begin{bmatrix} d_{x1} \\ d_{x2} \end{bmatrix} = \begin{bmatrix} F_1(t) \\ 0 \end{bmatrix} \quad (3)$$

where  $m_1$  and  $m_2$  is the sand mass and mass of the piston plate, respectively,  $k_1$  is the sand stiffness and  $k_2$  is the stiffness of the spring. Further  $\ddot{u}_1$  and  $\ddot{u}_2$  are accelerations and  $u_1$  and  $u_2$  are displacements of masses  $m_1$  and  $m_2$ , respectively. To correctly simulate that sand cannot transfer tensile forces the sand spring  $k_1$  was modified so that only compressive forces could be transferred; spring  $k_2$  though was linear elastic in both directions. The influence of damping was assumed to be negligible and is hence not included in the model.

The added mass of sand is calculated as the mass of a cone between the charge point and the piston plate (i.e. the base of the cone) as

$$m_{sand} = A \cdot \frac{r}{3} \cdot \rho_{sand} \quad (4)$$

where  $r$  is the charge distance of 1.0 m. This gives an added mass of  $m_{sand} = 51$  kg.

The linear spring stiffness of the sand is approximated as

$$k_{sand} = \frac{K_{sand} \cdot A}{l_{cone}} \quad (5)$$

where  $K_{sand}$  is the sand bulk modulus,  $A = 0,091$  m<sup>2</sup> is the area of the piston plate and

$$l_{cone} = \frac{r}{3} \quad (6)$$

is the distance from the centre point of the sand cone to the piston plate. The bulk modulus was approximated to be equal both for loading and unloading cases and was determined as

$$K_{sand} = \rho_{sand} \cdot c_{sand}^2 \quad (7)$$

where  $\rho_{sand} = 1674$  kg/m<sup>3</sup> is the sand in situ density, and  $c_{sand} = 350$  m/s is the speed of the pressure wave (a typical value for dry soil [14]). This gives a sand bulk modulus of  $K_{sand} = 0.2$  GPa (as a comparison, this is about ten times smaller than the bulk modulus of water,  $K_{water} = 1000 \cdot 1484^2 = 2.2$  GPa) and a sand spring stiffness  $k_{sand} = 54.6$  MN/m. The model parameters are gathered in Table 1.

Table 1 Model parameters used in the 2DOF model.

Case	Structure		Added sand		Load Condition	
	$m$ [kg]	$k$ [MN/m]	$m_{sand}$ [kg]	$k_{sand}$ [MN/m]	$F(t)$ [N]	$v_0$ [m/s]
1	5.2	0.1 - 1.2	51	54.6	-	1.5
2	10.6	0.1 - 1.2	51	54.6	-	1.5
3	24.7	0.1 - 1.2	51	54.6	-	1.5
4	58.8	0.1 - 1.2	51	54.6	-	1.5

In this paper the hypothesis was that an effective sand mass, with an initial particle velocity, will give satisfying agreement with the FE-results of initial reflected pressure for the different piston masses and spring deflection. The initial particle velocity used was derived with ConWep [20] for a sand with 1674 kg/m<sup>3</sup> in density, seismic wave speed 350 m/s, and 2.75 attenuation factor, this gave a particle velocity of 1.5 m/s for the selected charge size and distance to target.

## EXPERIMENTAL RESULTS

When the piston mass was varied [5.2, 10.6, 24.7, and 58.8 kg], and the spring stiffness was kept constant to  $k = 0.1 \text{ MN/m}$ , pressures according to Fig. 8 is shown in [3]. The dotted and dashed lines in Fig. 8 represent the estimated inertial pressure  $P_{inertia}$  and spring pressure  $P_{spring}$ , defined as shown in equations (8) and (9), respectively. From this it can be noted that the dotted line in Fig. 8 follows well the measured pressure for the first peak; i.e. the first peak of the reflected pressure directly depends on the piston mass.

$$P_{inertia}(t) = \frac{m \cdot a_x(t)}{A} \quad (8)$$

$$P_{spring}(t) = \frac{k \cdot d_x(t)}{A} \quad (9)$$

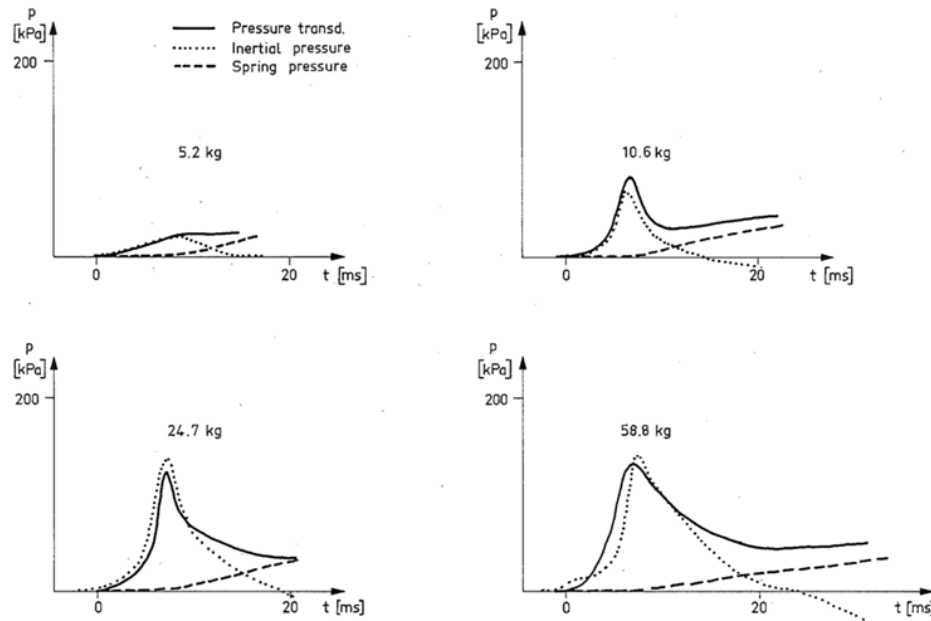


Fig. 8 Measured reflected pressure, estimated mass inertia pressure, and spring pressure for various piston masses and with constant spring stiffness  $k = 0.1 \text{ MN/m}$ . From [3].

For the results in Fig. 9 the spring stiffness was increased to  $k = 0.5 \text{ MN/m}$  while the piston mass was varied [5.2, 10.6, 24.7, and 58.8 kg]. Now it can be noticed that the second ridge of the reflected pressure is more dominant in Fig. 9 compared to that in Fig. 8. A clear first peak is not visible until the piston mass reaches 24.7 kg. In Fig. 10 the reflected pressures from Fig. 9 are merged into one plot and from this it can be seen that when the piston mass was 5.2 and 10.6 kg there is no clear first peak in the reflected pressure. However, all of them have a second ridge in the reflected pressure, even though it is slightly reduced when the piston mass is increased.



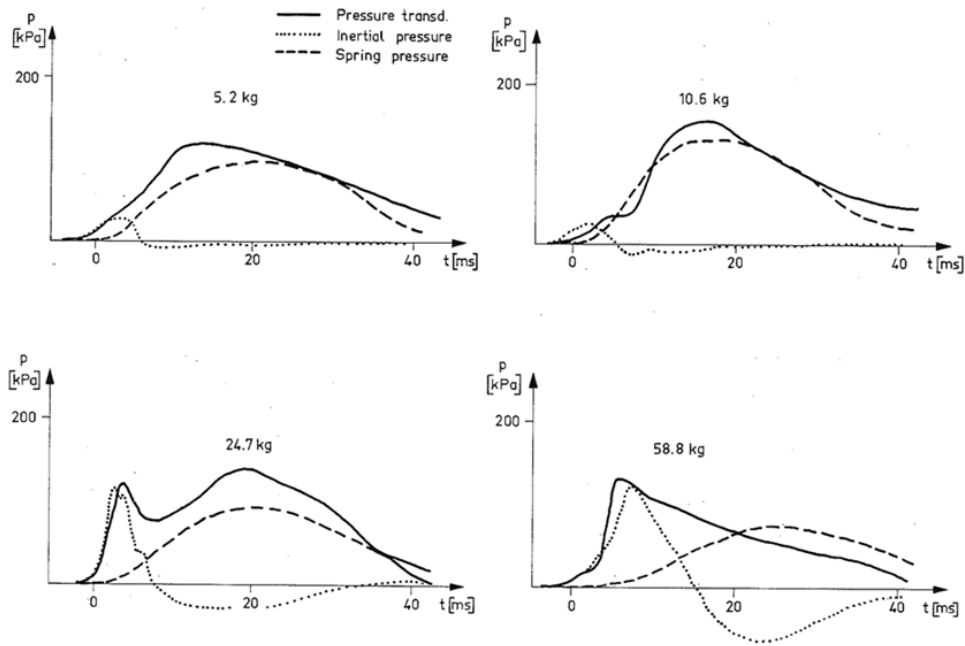


Fig. 9 Measured reflected pressure, estimated mass inertia pressure, and spring pressure for various piston masses and with constant spring stiffness 0.5 MN/m. From [3].

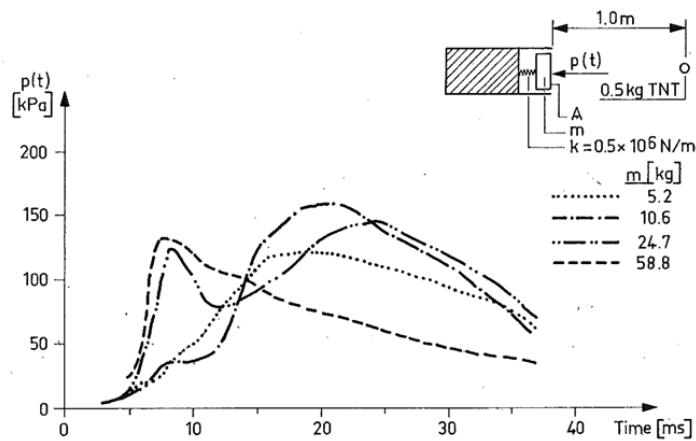


Fig. 10 Comparison of measured reflected pressure from Fig. 9 for various piston masses and with constant spring stiffness 0.5 MN/m. From [3].

In Fig. 11a the piston mass was kept constant at  $m = 5.2$  kg while the spring stiffness was varied [0.1, 0.5, 1.2 MN/m]. It shows clearly how the second ridge increases with the spring stiffness. In comparison, the reflected pressure is shown in Fig. 11b when the piston mass is high,  $m = 58.8$  kg, while the spring stiffness was varied [0.1, 0.5, 1.2 MN/m]. From this it can instead be noted that the pressure is only very little affected by the stiffness; i.e. the reflected pressure is mainly governed by the piston mass. Still the trend with the second ridge increases somewhat with increasing spring stiffness.

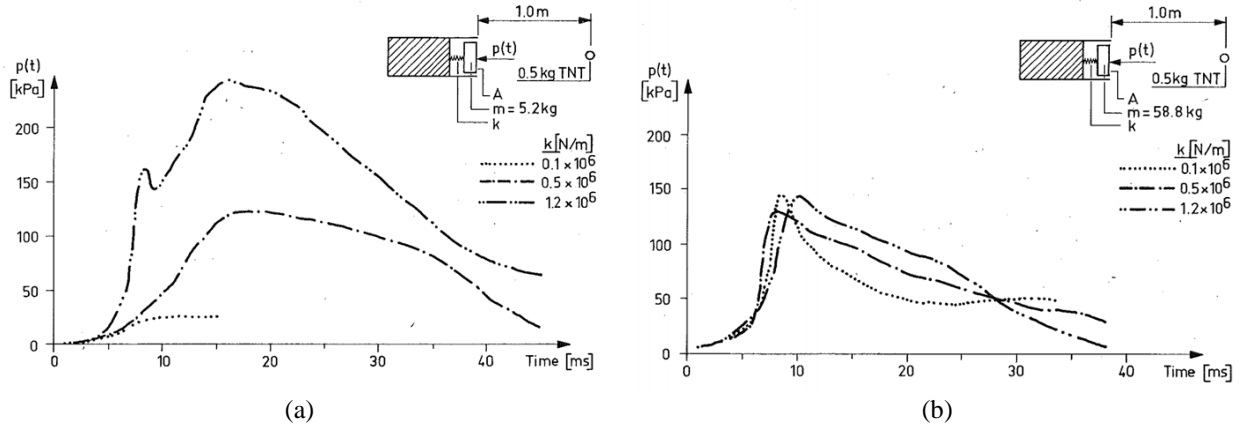


Fig. 11 Measured reflected pressures for varied spring stiffness and with constant piston mass: (a) 5.2 kg, and (b) 58.8 kg. From [3].

### SIMULATION RESULTS

Simulations have been carried out for a total of twelve combinations of different piston mass [5.2, 10.6, 24.7, 58.8 kg] and spring stiffness [0.1, 0.5, 1.2 MN/m]. One case was studied in more in-depth, 24.7 kg and 0.5 MN/m. Consequently, a lot of results are obtained but in order to save space mainly the response of low ( $m = 5.2$  kg), medium ( $m = 24.7$  kg), and high piston mass ( $m = 58.8$  kg) is shown and discussed. The total simulated time was 50 ms.

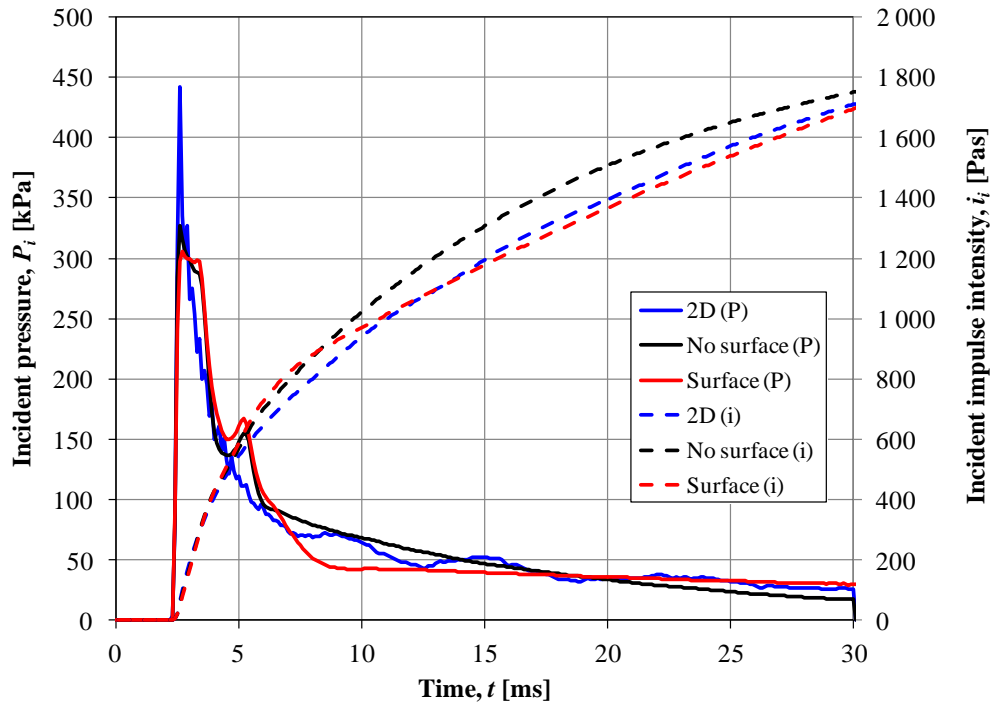


Fig. 12 Free field incident pressure when no ground surface is modelled and compared with simulation of case shown in Fig. 4 when the cylinder is not present. [Free field 2D:  $P_i = 442$  kPa,  $i_i = 1711$  Pas; Free field 3D:  $P_i = 327$  kPa,  $i_i = 1750$  Pas; Free field with ground surface:  $P_i = 305$  kPa,  $i_i = 1694$  Pas].

The 3D free field incident pressure of the simulations is compared with a model with ground surface reflections at the intended depth of and position of the piston, Sjöbo material model, are shown in Fig. 12. These pressures correspond to a case when the cylinder is not present; i.e. no reflection pressure. If the ground shock wave is reflected against a fully rigid surface, the pressure is increased with a factor of 2. However, as is shown further down, this is not the case in the simulations carried out here. From Fig. 12 it can be seen that the presence of a nearby free ground surface has some influence of the resulting  $P(t)$  relation; after about 7 ms the pressure drops for the case with modelled surface compared to the case with no surface modelled. This is believed to be an effect of the relief wave caused by the free ground surface. The same shape was also confirmed by ConWep [13] when tensile surface reflections are included. Consequently, the impulse intensity also decrease, although it can be noted that the overall difference between the two cases are relatively limited. Hence, this indicates that the charge burial depth chosen by Hultgren in his experiments was well chosen to limit the effect of the free surface. The impulse intensity between free-field in 2D and 3D is similar but for the peak pressure, though, there is a difference (442 kPa vs 327 kPa). This difference is related to different mesh size used (2D: 5 mm; 3D: 20 mm); a larger mesh size was used in the 3D models to achieve reasonable computational times.

In the FE simulations the reflected pressure  $P_r(t)$  of the piston was determined as

$$P_r(t) = \frac{m \cdot a_x(t) + k \cdot d_x(t)}{A} \quad (10)$$

and in Fig. 13 the variation of this pressure is shown in for a case with intermediate spring stiffness ( $k = 0,5 \text{ MN/m}$ ), and two different masses. From this it can be noted that a distinct initial pressure spike appear and that it is considerably larger when the mass is high ( $m = 58.8 \text{ kg}$ ) compared to when the mass is low ( $m = 5.2 \text{ kg}$ ). The appearance of such a distinct pressure peak is not in conjunction with the experiments; compare reflected pressure in Fig. 8 and Fig. 9. Further, the forming of a first (inertia) and second pressure peak (stiffness), as schematically illustrated in Fig. 2, is not as clearly identified in the simulations as in the experiments. This is believed to at least partly be to the strong initial pressure peak obtained in the simulations. The reason for the above discrepancies are unknown. However, measuring the pressure in a material like sand is difficult and one possible reason might be that such an initial peak may have been filtrated away in the original experiments. The difference is noted but is apart from that not further treated here.

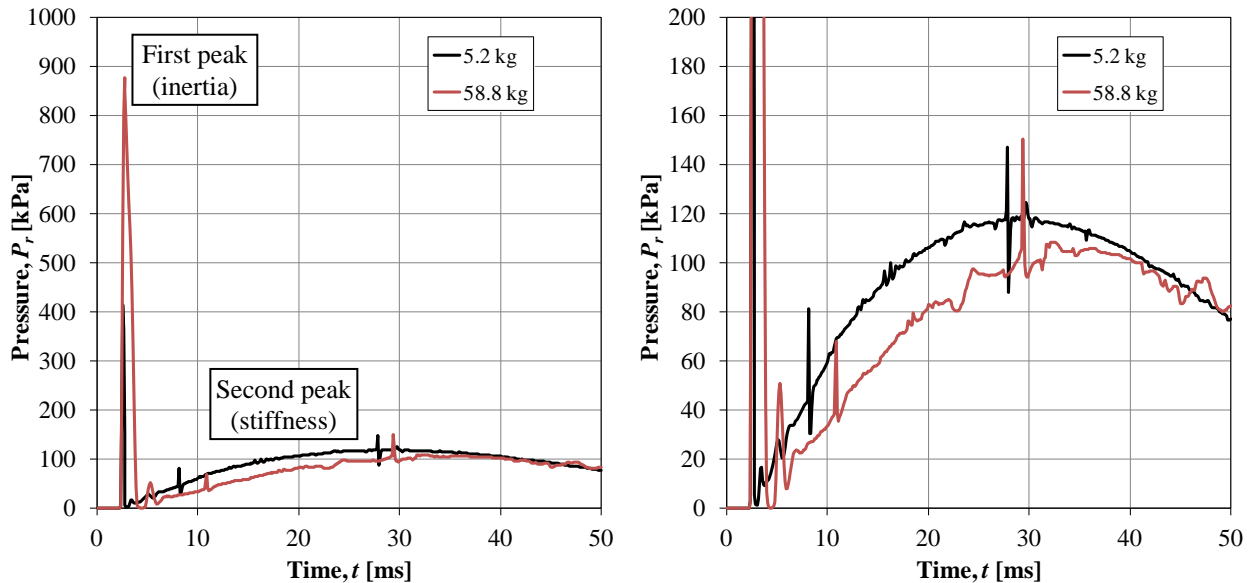


Fig. 13 Comparison of simulated reflected pressure for various piston masses and with constant spring stiffness 0.5 MN/m. Note that both diagrams show the same result but with different scaling on the vertical axis. For the corresponding experimental results, see Fig. 10.

Here, the first mass inertia phase of the reflected pressure is analyzed in further detail before continuing discussing the complete spring deflection and reflected pressure of the full simulated time of 50 ms. The initial phase, up to 5 ms, of the ground shock's influence on the reflecting pressure and particle velocity in the piston plate and sand, is compared in Fig. 14. The solid lines in Fig. 14a show the reflected pressures calculated by equation (10) and the dashed lines shows the measured soil pressure and particle velocity, respectively, in the euler cell located next to the axis symmetric line, 0.01 m in-front of the piston surface. Here it can be seen that the reflected pressure is a direct function of the piston mass. In Fig. 14b it is shown how the piston velocity is accelerated (solid lines) and the dashed lines shows the particle velocity in the soil which shows that the piston and soil is moving with approximately the same velocity. The soil velocity is mostly higher than the piston velocity, which indicates that the soil mass seems to have more kinetic energy and continue to push the piston forward.

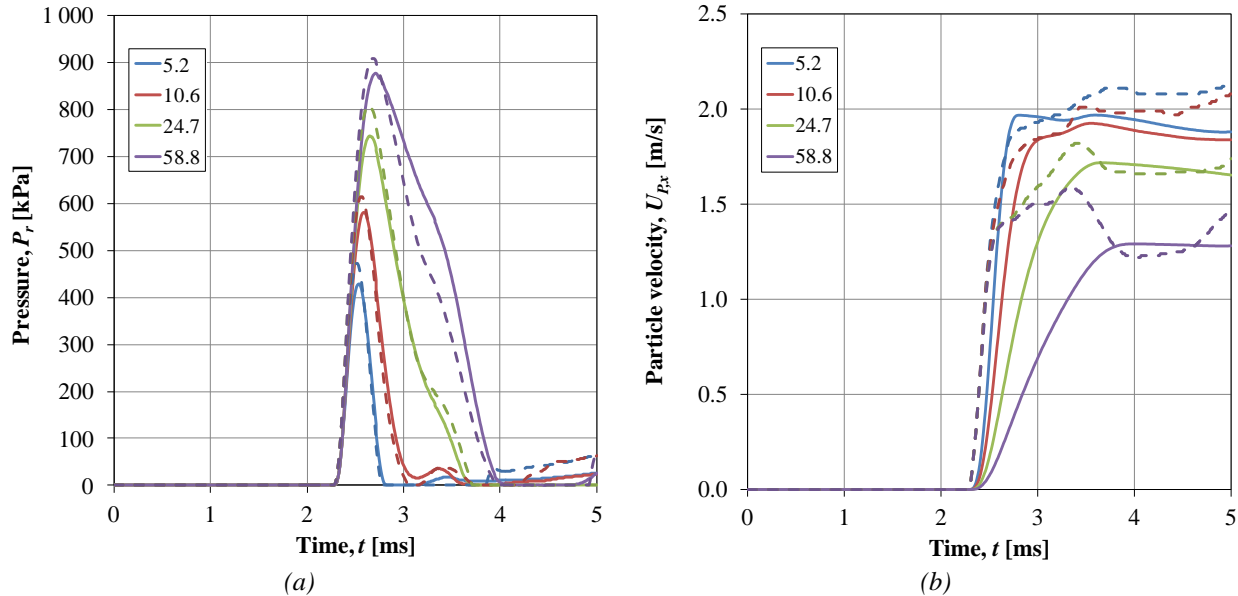


Fig. 14 Comparison of (a) reflected initial pressure, and (b) particle velocity at piston (solid) and just outside piston in soil (dashed) as function of time up to 5 ms for various masses and spring stiffness 0.5 MN/m.

This concept of first initial reflected pressure effect due to piston mass inertia can be modelled by a 2DOF model. As previously described the soil mass and soil stiffness are approximated to be 51 kg and 54.6 MN/m, respectively, and the initial velocity of the sand, at which it impacts the piston plate, was determined based on ConWep [20] and is confirmed by the particle velocity shown in Fig. 15. Here the velocity, at a distance of 0.01 m from the piston plate, is used at the moment in time that corresponds to the maximum reflected pressure shown in Fig. 14a; i.e.  $U_{p,x} = 1.5$  m/s. At time  $t = 2.2$  ms there is a remapping of results from 1D wedge model to the full 3D model, and hence there is a minor discontinuity in Fig. 15 due to change of mesh size.

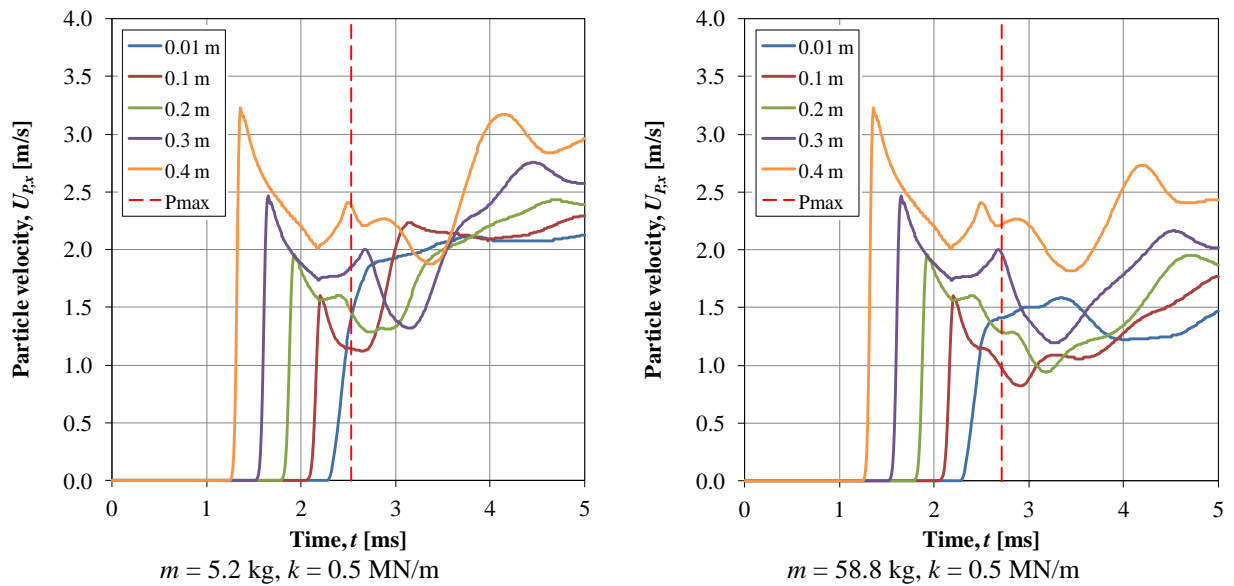


Fig. 15 Particle velocity in sand at various distances from the front of the piston plate. The dashed red line marks the moment in time when the maximum reflected pressure is obtained against the piston plate. Close to the piston plate (distance 0.01 m) the particle velocity is about 1.5 m/s when the pressure peak is reached.

The plots in Fig. 16 shows the pressure wave propagation between 2.3 ms and 3.5 ms for the 24.7 kg piston with spring stiffness 0.5 MN/m. Quite early on, the initial reflected pressure wave due to mass inertia is over. As expected the load duration increased with increased piston mass.

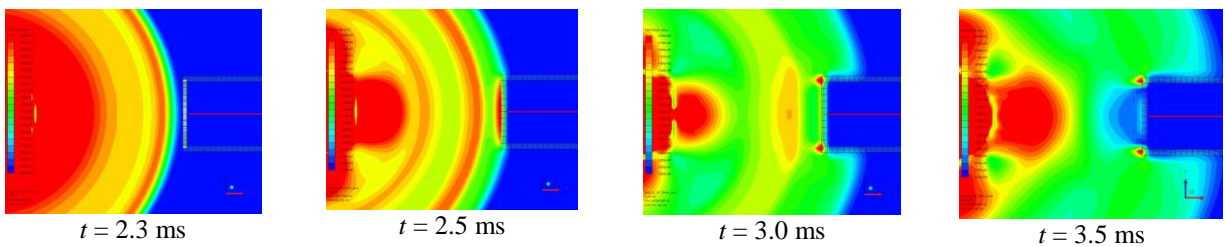


Fig. 16 Pressure plots at different times from 3D FE simulations of 24.7 kg and 0.5 MN/m spring. Blue = 0 kPa, green = 300 kPa, red  $\geq 600$  kPa.

Fig. 17 shows the shear stress (ZX) for the same times as in Fig. 16 and indicates that in front of the piston there is a cone shaped part which can be seen at 3 ms. This can be an indication that there is an effective sand mass, i.e. the sand inside the cone, colliding with the piston at a certain particle velocity.

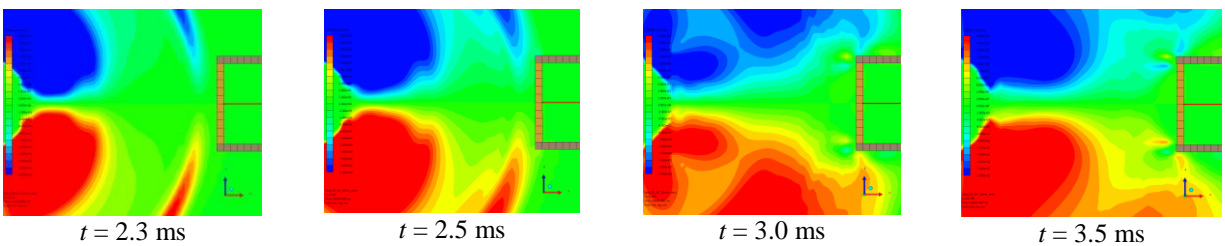


Fig. 17 Shear stress (ZX) plots at different times from 3D FE simulations of 24.7 kg and 0.5 MN/m spring. Blue  $\leq -200$  kPa, green = 0 kPa, red  $\geq 200$  kPa.

In Fig. 18, the corresponding sand particle velocity  $U_{p,x}$  is shown. Here it can be noted that the sand velocity directly in front of the piston is larger compared to the sand outside the piston; i.e. these plots indicate that the sand in front of the projected area of the piston encounter less resistance compared to the sand outside this zone. This response is conceptually very different to that obtained in e.g. a shock wave in air hitting an object, in which the shock wave in front of the object instead would reflect against it. The shape of the velocity distribution differs from the cone shaped part found in Fig. 17, which indicates that the volume of the effective mass colliding with the piston may be larger than that assumed in the 2DOF simulations. It seems like the mass transportation away from the centre of the explosive detonation increases in the direction of the piston, and the simple reason is that the spring stiffness for the piston is maximum 1.2 kN/m which is over 45 times smaller than the sand stiffness 54.6 kN/m, see Table 1. This gives the sand a reason to transport more sand material towards the piston area which is more easily compressed than the surrounding sand.

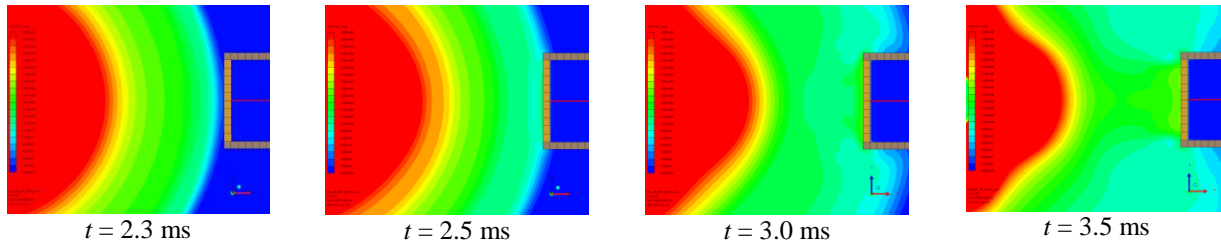


Fig. 18 Absolute velocity plots at different times from 3D FE simulations of 24.7 kg and 0.5 MN/m spring. Blue  $\leq 0$  m/s, green = 1.5 m/s, red  $\geq 3$  m/s.

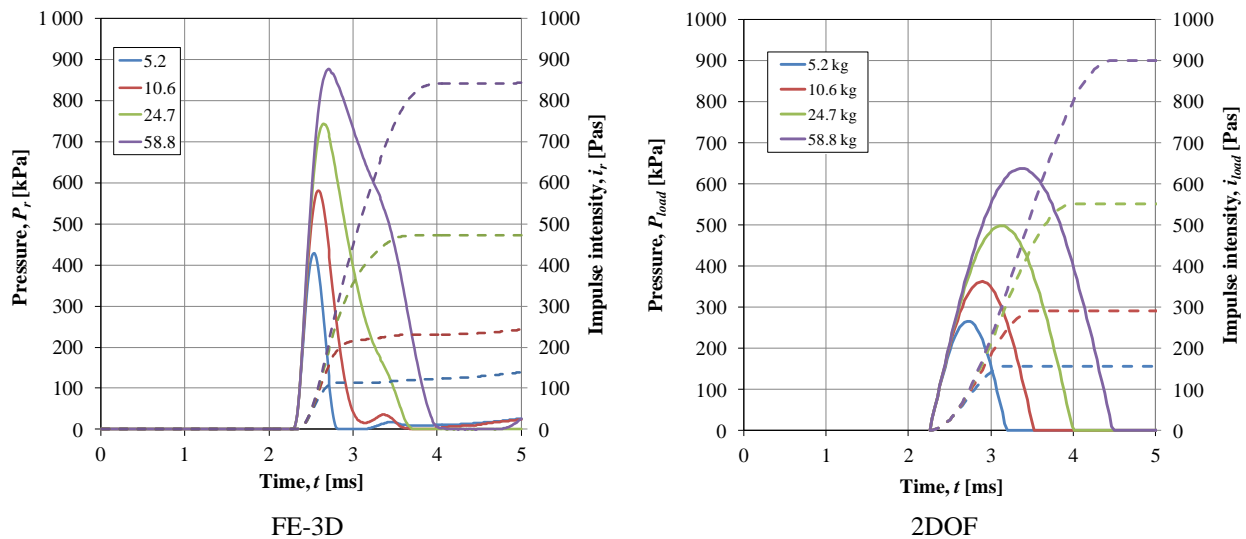


Fig. 19 Comparison of initial reflected pressure (solid lines) and reflected impulse intensity (dashed lines) obtained from 3D FE analyses and 2DOF model for various masses and spring stiffness 0.5 MN/m.

The resulting pressure from the 2DOF analysis, acting between sand cone and piston plate, is in Fig. 19 compared with the reflected pressure acting on the piston plate in the 3D FE analyses for different masses when  $k = 0.5$  MN/m. No influence on the resulting pressure was obtained due to spring stiffness; and hence it can be observed that for the cases studied the initial reflected pressure only depends on the piston mass.

The reflected pressure, and, initial impulse intensity for 3D-FE simulations and 2DOF model is shown in Table 2. The 2DOF model underestimate the peak pressure by an average ratio of 0.67 when comparing with 3D FE simulations. This difference is believed to be due to an underestimation of the sand spring stiffness  $k_I$  when using the 2DOF model. However, the impulse intensity of the first 4 ms has an average ratio of 1.21, where 2DOF overestimates, and this difference is deemed to be acceptable when one is comparing the computational effort of FE-3D and 2DOF needed to achieve these results.

If the incident wave was reflected against a rigid surface the pressure should increase with a factor of 2. Here, though, the ratio  $P_{load} / P_i$  is less than that; with decreasing value for decreasing piston mass. The reflection factor depends on the mass and varies from 0.97 to 1.98 (comparing with free field incident pressure in 2D) when the piston mass is increased from 5.2 to 58.8 kg, see Table 3. The 3D FE results lead to unrealistically low incident pressure, see Fig. 12, therefore the reflection factor of 2D FE results are only considered here.

Table 2 Relation between reflected pressure obtained in FE-3D analyses and 2DOF analyses.

Case	Structure		3D-FE		2DOF		Ratio, 2DOF / FE	
	$m$ [kg]	$k$ [MN/m]	$P_r$ [kPa]	$i_r^{1)}$ [Pas]	$P_{load}$ [kPa]	$i_{load}^{1)}$ [Pas]	$P$ [-]	$i$ [-]
1	5.2	0.1	428	117	265	156	0.62	1.33
2	10.6	0.1	580	228	361	290	0.62	1.27
3	24.7	0.1	744	470	497	549	0.67	1.17
4	58.8	0.1	876	839	636	899	0.73	1.07
5	5.2	0.5	428	123	265	156	0.62	1.28
6	10.6	0.5	580	230	361	291	0.62	1.26
7	24.7	0.5	744	471	497	550	0.67	1.17
8	58.8	0.5	876	840	636	900	0.73	1.07
9	5.2	1.2	428	132	265	157	0.62	1.19
10	10.6	1.2	580	235	362	292	0.62	1.24
11	24.7	1.2	744	473	497	551	0.67	1.17
12	58.8	1.2	876	841	636	901	0.73	1.07
Average							0.67	1.21

<sup>1)</sup> The given impulse intensity corresponds to the initial pressure peak and is determined at time  $t = 4$  ms.

Table 3 Relation between reflected pressure and incident pressure in simulations.  
[Free field 2D:  $P_{i,2D} = 442$  kPa; Free field 3D:  $P_{i,3D} = 327$  kPa]

Case	Structure		Reflection factor		
	$m$ [kg]	$k$ [MN/m]	$P_r$ [kPa]	$P_r / P_{i,2D}$ [-]	$P_r / P_{i,3D}$ [-]
1	5.2	0.1 - 1.2	428	0.97	1.31
2	10.6	0.1 - 1.2	580	1.31	1.77
3	24.7	0.1 - 1.2	744	1.68	2.27
4	58.8	0.1 - 1.2	876	1.98	2.68

Simulation results for low ( $m = 5.2$  kg) and high piston mass ( $m = 58.8$  kg) for low ( $k = 0.1$  MN/m), intermediate ( $k = 0.5$  MN/m) and high stiffness ( $k = 1.2$  MN/m) are shown in Fig. 20 where reflected pressure, mass inertia pressure, and spring pressure are presented.

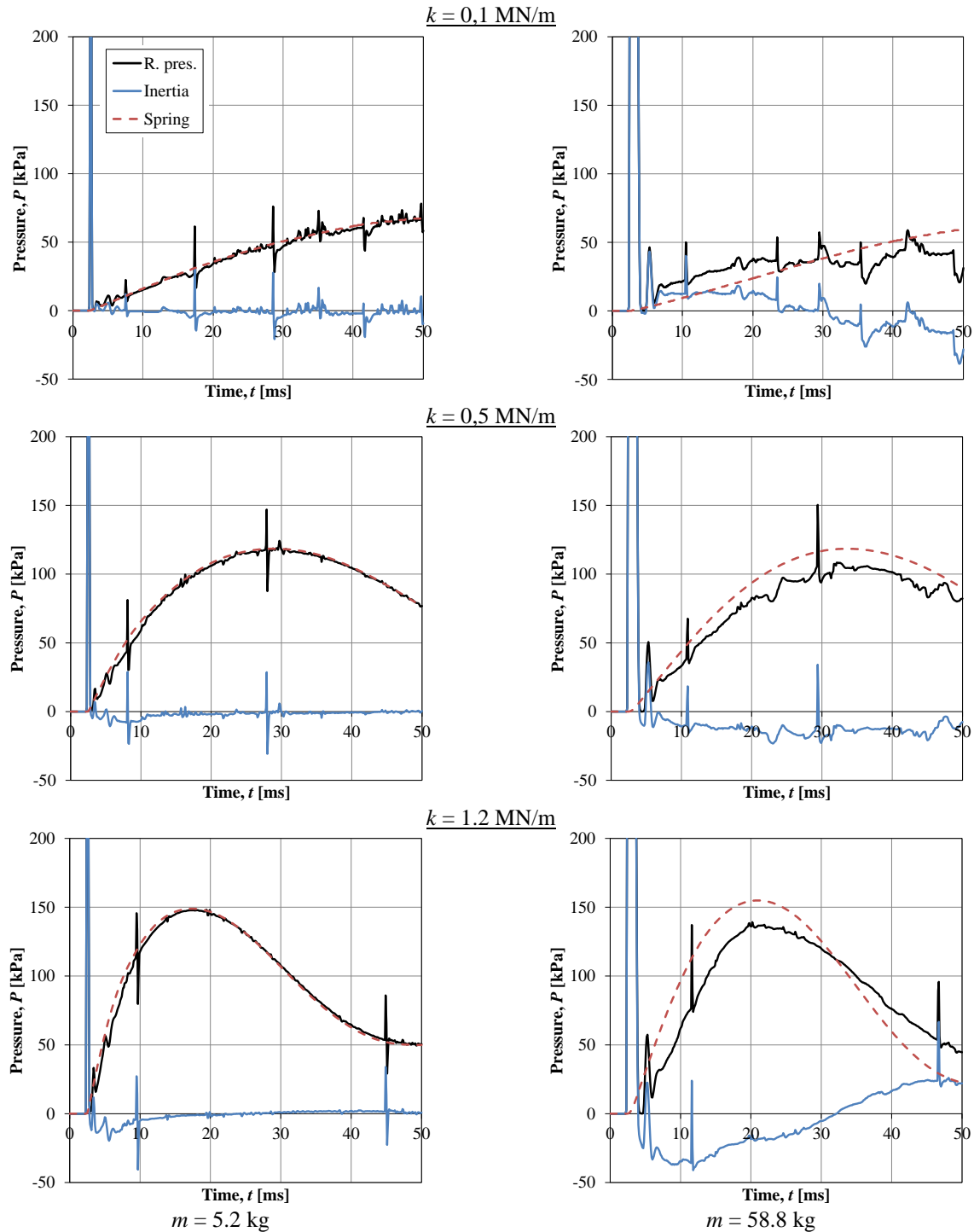


Fig. 20 Simulated reflected pressure, and calculated mass inertia pressure and spring pressure for various piston masses and spring stiffness 0.1 MN/m. For the corresponding experimental results, see Fig. 8 and Fig. 9.

In Fig. 21 the reflected pressure from different simulations is compared for various stiffness when the piston mass is low ( $m = 5.2 \text{ kg}$ ) and high ( $m = 58.8 \text{ kg}$ ). When the piston mass is low it is clear that increased stiffness result in an increase of the second pressure peak (stiffness) in a way that is in good conceptual agreement with the experiments, compare with Fig. 11a. When the piston mass is high, though, the resulting reflected pressure is not as straightforward. Here, the simulations show that an increased stiffness still has a similar effect and thus cause an



increased reflected pressure of the second pressure peak. However, this is not in full conjunction with the experimental observations, see Fig. 11b, where it is indicated that the effect of the stiffness is less important on the reflected pressure in the second pressure peak when the piston mass is high.

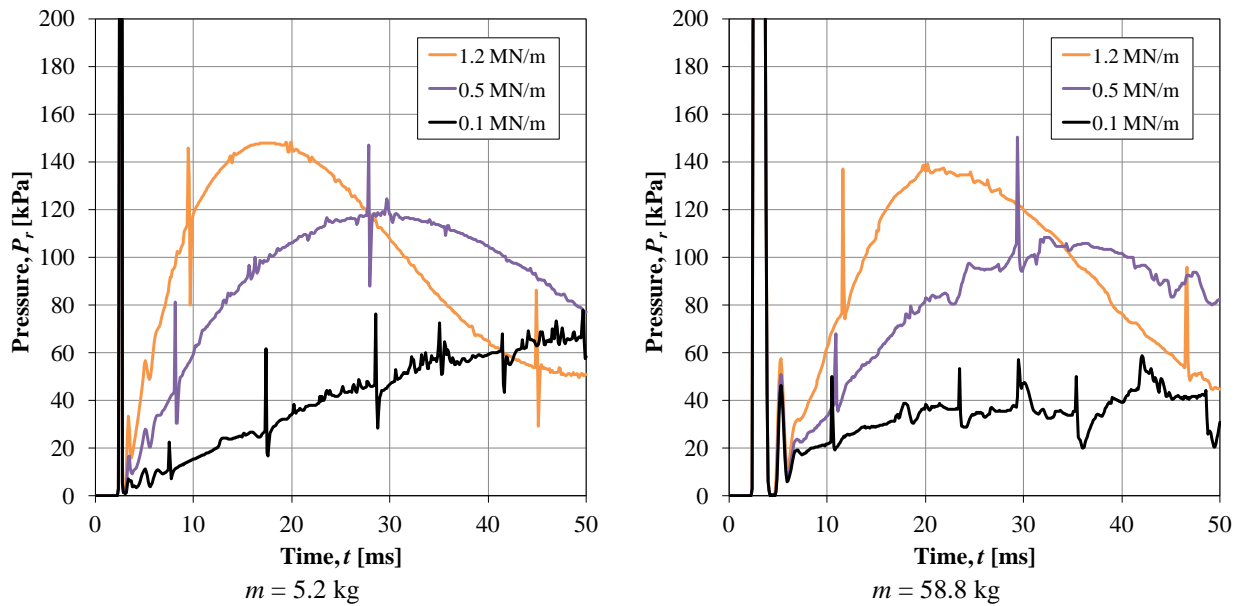


Fig. 21 Simulated reflected pressures for varied spring stiffness and with constant piston mass 5.2 kg or 58.8 kg. For the corresponding experimental results, see Fig. 11.

The structural response, i.e. the spring pressure, is less sensitive than that of the reflected pressure and mass inertia pressure. It is therefore of interest to compare the resulting spring pressures between both the simulations and the experiments. The maximum spring pressure calculated from experiments and simulations are listed in Table 4 and in Fig. 22 the spring pressure-time relations from the simulations (dashed lines) are shown for the possible combinations of four different piston masses and three different spring stiffnesses. For low ( $k = 0.1$  MN/m) and intermediate stiffness ( $k = 0.5$  MN/m) the calculated spring pressures from the experiments are shown as well with continuous lines.

Table 4 Comparison of maximum calculated spring pressure from experiments and simulations;  $P_{FE} / P_{exp}$  express the ratio between spring pressure from simulations and experiments.

Case	Structure		Experim. $P_{spring}$ [kPa]	FE-3D	
	$m$ [kg]	$k$ [MN/m]		$P_{spring}$ [kPa]	$P_{FE} / P_{exp}$ [-]
1	5.2	0.1	21	67	3.21
2	10.6	0.1	31	67	2.13
3	24.7	0.1	30	65	2.14
4	58.8	0.1	34	59	1.75
5	5.2	0.5	95	118	1.24
6	10.6	0.5	129	120	0.93
7	24.7	0.5	90	121	1.34
8	58.8	0.5	74	118	1.61
9	5.2	1.2	-	149	-
10	10.6	1.2	-	152	-
11	24.7	1.2	-	157	-
12	58.8	1.2	-	155	-

The occurrence of a strong initial peak pressure in the simulations also affects the responding spring pressure. Thus, the spring pressure in the simulations does not get a slow gradual increase, as was the case in the experiments, but rather a somewhat quicker raise in pressure; compare simulated and experimental results in Fig. 22. The trend of low spring stiffness leads to lower pressures both in simulations and experiments.

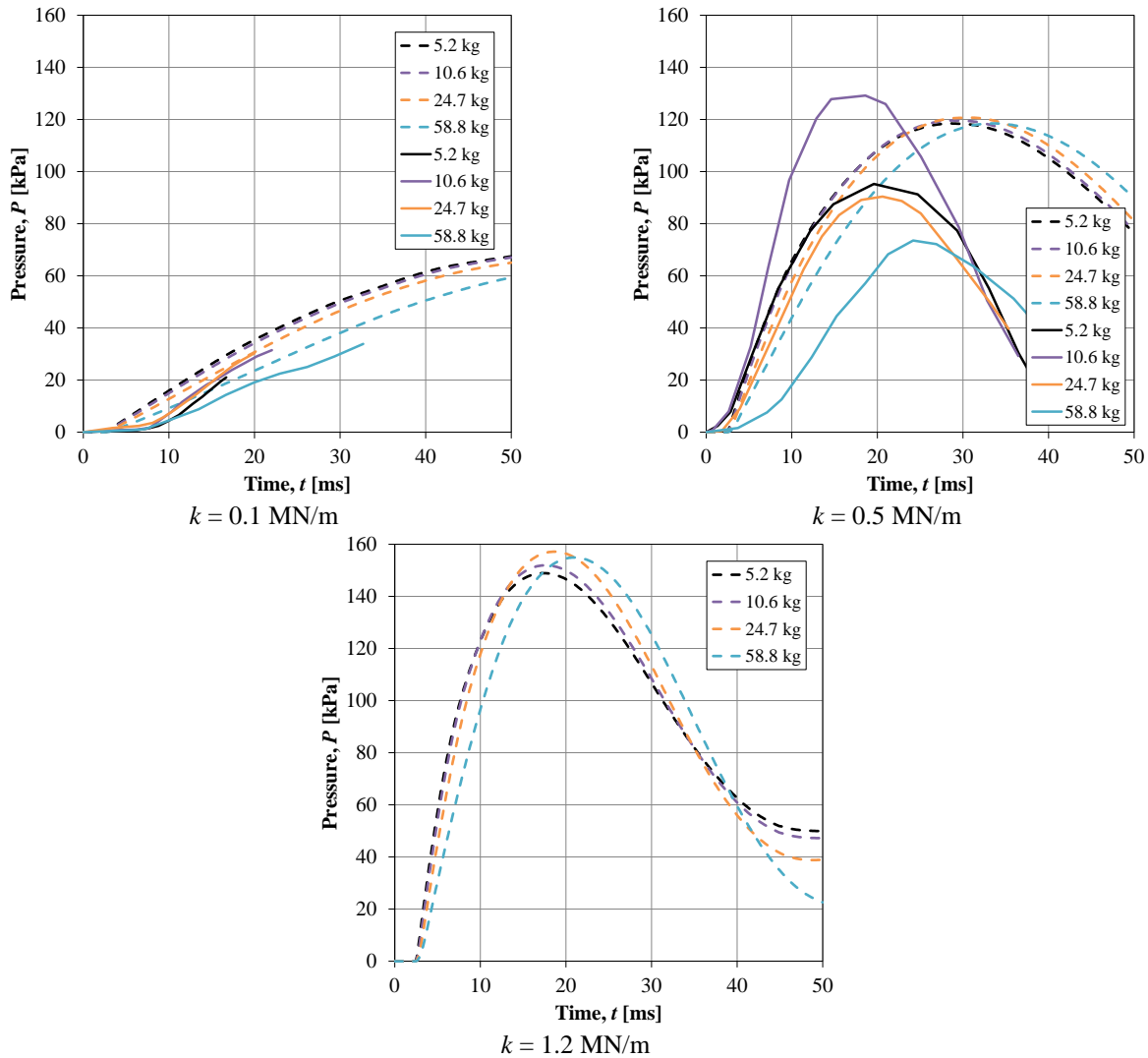


Fig. 22 Comparison of calculated spring pressure for varied spring stiffness and piston mass. Results from simulations in dashed lines; results from experiments in continuous lines.

## **CONCLUSIONS AND FUTURE WORK**

The principle of reflected pressure is built up by the reflected structure's mass inertia and spring stiffness pressure is confirmed. However, the experimental results lack the distinct pressure spike, due to inertia, occurring in the FE simulations during the first 4 ms. A reason for this discrepancy can be how the sand pressure measurement was conducted in the experiments; and it is believed that it is more correct that there will be a clear initial pressure peak, as is indicated in the simulations. The first mass inertia dependent part of reflected pressure was modelled rather accurately with a simplified 2DOF model. When comparing with 3D FE simulation results, the initial reflected peak pressure is unaffected by what spring stiffness the piston had. The initial conditions and parameters for the 2DOF model were quite simplified: an added mass for sand, calculated as a cone, initial particle velocity derived from ConWep, simple stiffness for sand only transferring compressive pressure, but this still gave reasonable results in average the pressure was underestimated with a factor 0.67 when using the 2DOF model, however the first four ms impulse intensity was overestimated by a factor 1.21 when using the 2DOF model, which is really acceptable results when comparing the computational efforts needed for the two methods. The total spring displacement needs further investigation when it comes to 2DOF analysis, as the particle velocity plots from 3D FE results indicate the total mass transportation seem to increase as a function of time which makes initial condition and loading conditions more complicated for the 2DOF model. Another way to understand the phenomena further is to systematically conduct a parameter study where charge distance, charge size, and piston area is changed and then study how this will affect the added sand mass for the 2DOF model.

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